Traffic Signal Warrant Study

The Intersection of NM 599\ Via Veteranos \ NM 599 West Frontage Road Connector Santa Fe, NM

Prepared by
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Signal Warrant Study

Location: NM 599 & Via Veteranos and W. Frontage Road Connector 4 legged intersection

City: Santa Fe

County: Santa Fe

NMDOT District: District Five

Report Prepared by: Javier A. Martinez, P.E.

Counts Performed By: NMDOT – District Five Traffic Section

Date of Counts and Times: 1/8/2015 and 1/15/2015; 7 AM to 10 AM, 11 AM to 2 PM,

& 3 PM to 6 PM. A total of 9 hours of traffic turning movement counts was collected for this study. The turning movement counts are presented in Appendix A.

Major Street: NM 599 is a 4-Lane Divided Rural Principal Arterial running Northbound and Southbound with a left turn and right turn deceleration lane and left turn acceleration lanes each direction at this intersection.

Minor Street: East side approach is Via Veteranos and the West side approach is the NM 599 West Frontage Road Connector.

I. General:

In order for a signal installation to be recommended at a New Mexico Department of Transportation (NMDOT) intersection, a signal warrant study has to be conducted. In accordance with the Manual on Uniform Traffic Control Devices (MUTCD), at least one of the following traffic signal warrants has to be met in order for the intersection to be eligible for consideration for the placement of a traffic signal. The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

The warrants are listed below:

Warrant 1, Eight-Hour Vehicular Volume.

Warrant 2, Four-Hour Vehicular Volume.

Warrant 3, Peak Hour.

Warrant 4, Pedestrian Volume.

Warrant 5, School Crossing.

Warrant 6, Coordinated Signal System.

Warrant 7, Crash Experience.

Warrant 8, Roadway Network

Warrant 9, Intersection near a Grade Crossing

A traffic control signal will not be installed unless the engineering study indicates that installing a traffic control signal will improve the overall safety and/or operation of the intersection. In accordance with the MUTCD, this study may consider the effects of the right-turn vehicles from the minor-street approaches. Engineering judgment should be used to determine what, if any, portion of the right-turn traffic is subtracted from the minor-street traffic count when evaluating the count against the above signal warrants. For this study, the right turn traffic on the minor streets were removed because the side streets provide an exclusive right turn lane and the movement enters the major street with minimal conflict. The side street approaches will be evaluated as one lane approaches with only the traffic volume in the through/left turn lane considered.

II. Purpose of Study:

The purpose of the traffic signal warrant study is to determine if the intersection of NM 599 and Via Veteranos / W. FR Connector, meets the necessary warrants to be considered by the NMDOT for placement of a traffic signal.

III. Location and Description:

NM 599 is a North/South 4-lane rural principal arterial roadway with two through lanes in each direction separated by a raised median with left turn and right turn deceleration lanes and left turn acceleration lanes in both directions. The East side approach is Via Veteranos and West side approach is W. FR Connector at this intersection.

The posted speed limit on NM 599 is 55 MPH.

The community of Santa Fe, NM has a population of more than 10,000. The state highway access category for NM 599 PRAR (Rural Principal Arterial) has a recommended signal spacing of 5280 feet based on the requirements of the *State Access Management Manual*. There are no traffic signals within a mile from the study intersection.

IV. Warrant Analysis:

A signal warrant analysis was performed for the intersection of NM 599\Via Veteranos\NM 599 W. Frontage Road Connector from traffic data collected on January 8 and January 15, 2015. The traffic counts are presented in Appendix A. For this study, the right turn volumes were eliminated from the minor street.

The following are the finding of the signal warrant analyses:

1) Warrant 1, Eight-Hour Vehicular Volume

The Minimum Vehicular Volume, Condition A, is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal.

The Interruption of Continuous Traffic, Condition B, is intended for application at locations where Condition A is not satisfied and where the traffic volume on a major street is so heavy that traffic on a minor intersecting street suffers excessive delay or conflict in entering or crossing the major street.

It is intended that Warrant 1 be treated as a single warrant. If Condition A is satisfied, then the criteria for Warrant 1 are satisfied and Condition B and the combination of Conditions A and B are not needed. Similarly, if Condition B is satisfied, then the criteria for Warrant 1 are satisfied and the combination of Conditions A and B is not needed.

The need for a traffic control signal shall be considered if an engineering study finds that one of the following conditions exist for each of any 8 hours of an average day:

- A. The vehicles per hour given in both of the 100 percent columns of Condition A in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to the intersection; or
- B. The vehicles per hour given in both of the 100 percent columns of Condition B in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to the intersection.

In applying each condition the major-street and minor-street volumes shall be for the same 8 hours. On the minor street, the higher volume shall not be required to be on the same approach during each of these 8 hours.

Combination of Warrants A& B:

The combination of Conditions A and B is intended for application at locations where Condition A is not satisfied and Condition B is not satisfied and should be applied only after an adequate trial of other alternatives that could cause less delay and inconvenience to traffic has failed to solve the traffic problems.

The need for a traffic control signal shall be considered if an engineering study finds that **both of the following conditions exist** for each of any 8 hours of an average day:

A. The vehicles per hour given in both of the 80 percent columns of Condition A in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to the intersection; and

B. The vehicles per hour given in both of the 80 percent columns of Condition B in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to the intersection.

These major-street and minor-street volumes shall be for the same 8 hours for each condition; however, the 8 hours satisfied in Condition A shall not be required to be the same 8 hours satisfied in Condition B. On the minor street, the higher volume shall not be required to be on the same approach during each of the 8 hours.

Table 4C-1. Warrant 1. Eight-Hour Vehicular Volume

	Conditi	on A—Mir	ılmum \	/ehicule	r Volum)			
	of lanes for n each approach	Vehicles (total	per hou of both			h mino	igher- r-stree	er hou volum et appo	e roach
Major Street	Minor Street	100%*	80%*	70%*	58%4	100%	80%	70%	56%*
1 2 or more 2 or more 1	1 1 2 or more 2 or more	500 600 600 500	400 480 480 400	350 420 420 350	280 336 336 280	150 150 200 200	120 120 160 160	105 105 140 140	84 84 112 112

	Condition	B—Intern	iption o	f Contin	nuous Tr	affic			
	of lanes for n each approach	Vehicles (total		r on maj approac		mino	igher r-stre	er hol volum et app ction c	ie roach
Major Street	Minor Street	100%*	80%	70%*	58%*	100%	80%	70%	56%
1 2 or more 2 or more	1 1 2 or more 2 or more	750 900 900 750	600 720 720 600	525 630 630 525	420 504 504 420	75 75 100 100	60 60 80	53 53 70 70	42 42 56 56

^{*} Basic minimum hourly volume.

Conclusion:

Warrant 1 was not met

For Condition A, the warrant volumes for a 2-lane approach on the major street and a 1-lane approach on the minor street entrance are 420 vph and 105 vph respectively. Zero hours met for 8 hours that were counted. The 70% factor was used since NM 599 has a speed limit over 40 MPH.

For **Condition B**, the warrant volumes for a 2-lane approach on the major street and a 1-lane approach on the minor street are 630 vph and 53 vph respectively. This target was

^{*} Used for combination of Conditions A and B after adequate trial of other remedial measures.

^{*} May be used when the major-street speed exceeds 70 km/h or exceeds 40 mph or in an isolated community with a population of less than 10,000,

May be used for combination of Conditions A and B after adequate that of other remedial measures when the majorstreet speed exceeds 70 km/h or exceeds 40 mph or in an isolated community with a population of less than 10,000.

met for 2 hours which is less than the 8 hours needed to satisfy the warrant. The 70% factor was used.

Similarly the combination warrant for Conditions A or B were met for 5 hours which is less than the 8 hours needed to satisfy the warrant. The 56% factor was used. See Highway Capacity Software Warrants Summary in Appendix B.

2) Warrant 2, Four-Hour Vehicular Volume

The Four-Hour Vehicular Volume signal warrant conditions are intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic control signal. The need for a traffic control signal shall be considered if an engineering study finds that, for each of any 4 hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) all fall above the applicable curve in Figure 4C-1 for the existing combination of approach lanes. On the minor street, the higher volume shall not be required to be on the same approach during each of these 4 hours.

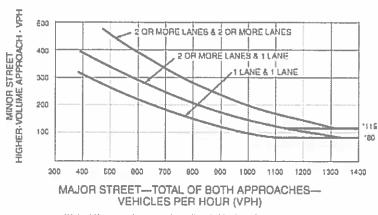
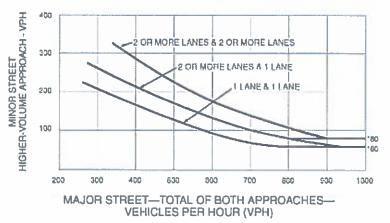


Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume

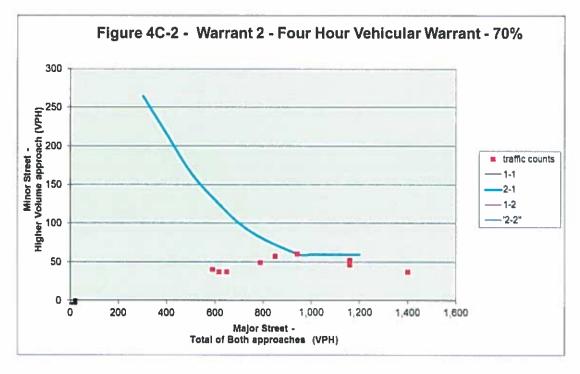
"Note: 115 vpti applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane."

Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)
[COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 70 km/h OR ABOVE 40 mph ON MAJOR STREET]



*Note: 80 vph applies as the lower threshold volume for a minor street approach with two or more tanes and 60 vph applies as the tower threshold volume for a minor-street approach with one lane.

Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.



Conclusion:

Warrant 2 was not met for 70% factor above 40 MPH 2 and 1 lane approach (figure: 4C-2).

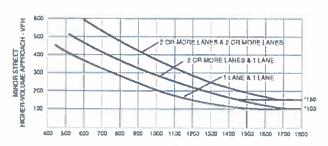
3) Warrant 3, Peak Hour

The Peak Hour signal warrant is intended for use at a location where traffic conditions are such that for a minimum of 1 hour of an average day, the minor-street traffic suffers undue delay when entering or crossing the major street. This signal warrant shall be applied only in unusual cases, such as office complexes, manufacturing plants, industrial complexes, or high-occupancy vehicle facilities that attract or discharge large numbers of vehicles over a short time.

The need for a traffic control signal shall be considered if an engineering study finds that the criteria in either of the following two categories are met:

- A. If all three of the following conditions exist for the same 1 hour (any four consecutive 15-minute periods) of an average day:
 - 1. The total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a STOP sign equals or exceeds: 4 vehicle-hours for a one-lane approach; or 5 vehicle-hours for a two-lane approach, and
 - 2. The volume on the same minor-street approach (one direction only) equals or exceeds 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes, and
 - 3. The total entering volume serviced during the hour equals or exceeds 650 vehicles per hour for intersections with three approaches or 800 vehicles per hour for intersections with four or more approaches.
- B. The plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) for 1 hour (any four consecutive 15-minute periods) of an average day falls above the applicable curve in Figure 4C-3 for the existing combination of approach lanes.

Figure 4C-3, Warrant 3, Peak Hour

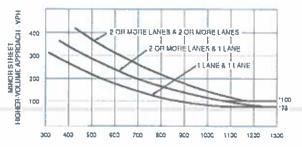


MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

Note: (50 vph applies as the lower threshold voture for a minor street approach with two or more laines and 100 vph applies as the lower threshold volume for a maner street approach with one lains.

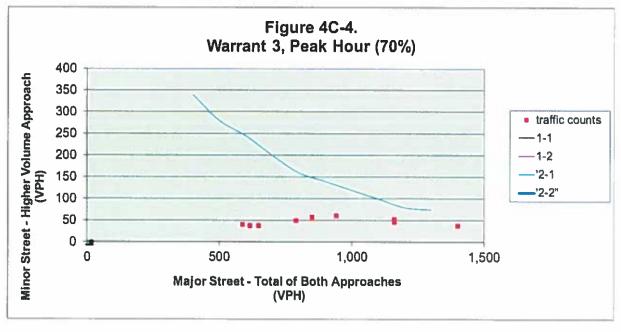
Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 70 km/h OR ABOVE 40 mph ON MAJOR STREET)



MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

"Note: 100 vph opplies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.



Conclusion:

Warrant 3 was not met for 70% factor above 40 MPH for a 2 and 1 lane approach (figure: 4C- 4) and is not considered an unusual case as described by MUTCD.

4) Warrant 4, Pedestrian Volume

The Pedestrian Volume signal warrant is intended for application where the traffic volume on a major street is so heavy that pedestrians experience excessive delay in crossing the major street.

The need for a traffic control signal at an intersection or midblock crossing shall be considered if an engineering study finds that both of the following criteria are met:

A. The pedestrian volume crossing the major street at an intersection or midblock location during an average day is 100 or more for each of any 4 hours or 190 or more during any 1 hour; and there are fewer than 60 gaps per hour in the traffic stream of adequate length to allow pedestrians to cross during the same period when the pedestrian volume criterion is satisfied. Where there is a divided street having a median of sufficient width for pedestrians to wait, the requirement applies separately to each direction of vehicular traffic.

The Pedestrian Volume signal warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 90 m (300 ft), unless the proposed traffic control signal will not restrict the progressive movement of traffic.

If this warrant is met and a traffic control signal is justified by an engineering study, the traffic control signal shall be equipped with pedestrian signal heads conforming to requirements set forth in the MUTCD.

Conclusion:

Warrant 4 was not met.

From observation, there were no pedestrian volumes at this intersection.

5) Warrant 5, School Crossing

The School Crossing signal warrant is intended for application where the fact that school children cross the major street is the principal reason to consider installing a traffic control signal.

The need for a traffic control signal shall be considered when an engineering study of the frequency and adequacy of gaps in the vehicular traffic stream as related to the number and size of groups of school children at an established school crossing across the major street shows that the number of adequate gaps in the traffic stream during the period when the children are using the crossing is less than the number of minutes in the same period (see Section 7A.03 of the MUTCD) and there are a minimum of 20 students during the highest crossing hour.

Before a decision is made to install a traffic control signal, consideration shall be given to the implementation of other remedial measures, such as warning signs and flashers, school speed zones, school crossing guards, or a grade-separated crossing.

The School Crossing signal warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 90 m (300 ft), unless the proposed traffic control signal will not restrict the progressive movement of traffic.

Conclusion:

Warrant 5 was not met.

There are no school crossings along NM 599.

6) Warrant 6, Coordinated Signal System

Progressive movement in a coordinated signal system sometimes necessitates installing traffic control signals at intersections where they would not otherwise be needed in order to maintain proper platooning of vehicles.

The need for a traffic control signal shall be considered if an engineering study finds that one of the following criteria is met:

- A. On a one-way street or a street that has traffic predominantly in one direction, the adjacent traffic control signals are so far apart that they do not provide the necessary degree of vehicular platooning.
- B. On a two-way street, adjacent traffic control signals do not provide the necessary degree of platooning and the proposed and adjacent traffic control signals will collectively provide a progressive operation.

Conclusion:

Warrant 6 was not met.

There is no coordinated signal system on this route. Therefore, this warrant is not satisfied.

7) Warrant 7, Crash Experience

The Crash Experience signal warrant conditions are intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal.

The need for a traffic control signal shall be considered if an engineering study finds that all of the following criteria are met:

- A. Adequate trial of alternatives with satisfactory observance and enforcement has failed to reduce the crash frequency; and
- B. Five or more reported crashes, of types susceptible to correction by a traffic control signal, have occurred within a 12-month period, each crash involving personal injury

- or property damage apparently exceeding the applicable requirements for a reportable crash; and
- C. For each of any 8 hours of an average day, the vehicles per hour (vph) given in both of the 80 percent columns of Condition A in Table 4C-1 (see Section 4C.02), or the vph in both of the 80 percent columns of Condition B in Table 4C-1 exists on the major-street and the higher-volume minor-street approach, respectively, to the intersection, or the volume of pedestrian traffic is not less than 80 percent of the requirements specified in the Pedestrian Volume warrant. These major-street and minor-street volumes shall be for the same 8 hours. On the minor street, the higher volume shall not be required to be on the same approach during each of the 8 hours

Conclusion:

Warrant 7 was not met.

Condition A is met as intersection ahead flashers have been installed. Condition B was not met with the Crash history gathered for the years; 2011, 2012 & 2013. There were seven reported accidents at the subject intersection, however none of the crashes would be susceptible to correction by the addition of a traffic signal. We are in the process of gathering reported crashes for 2014. Condition C was not met for the 8 hours required to satisfy this warrant. See Appendix E for crash data reports.

8) Warrant 8, Roadway Network

Installing a traffic control signal at some intersections might be justified to encourage concentration and organization of traffic flow on a roadway network.

The need for a traffic control signal shall be considered if an engineering study finds that the common intersection of two or more major routes meets one or both of the following criteria:

- A. The intersection has a total existing, or immediately projected, entering volume of at least 1,000 vehicles per hour during the peak hour of a typical weekday and has 5-year projected traffic volumes, based on an engineering study, that meet one or more of Warrants 1, 2, and 3 during an average weekday; or
- B. The intersection has a total existing or immediately projected entering volume of at least 1,000 vehicles per hour for each of any 5 hours of a normal business day (Saturday or Sunday).

A major route as used in this signal warrant shall have one or more of the following characteristics:

- A. It is part of the street or highway system that serves as the principal roadway network for through traffic flow; or
- B. It includes rural or suburban highways outside, entering, or traversing a City; or
- C. It appears as a major route on an official plan, such as a major street plan in an urban area traffic and transportation study.

Conclusion:

Warrant 8 is not met.

9) WARRANT 9, Intersection Near a Grade Crossing

The Intersection Near a Grade Crossing signal warrant is intended for use at a location where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic control signal.

The need for a traffic control signal shall be considered if an engineering study finds both of the following criteria are met:

- A. A grade crossing exists on an approach controlled by a STOP or YIELD sign and the center of the track nearest to the intersection is within 140 feet of the stop line or yield line on the approach; and
- B. During the highest traffic volume hour during which rail traffic uses the crossing, the plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the minor-street approach that crosses the track (one direction only, approaching the intersection) falls above the applicable curve in Figure 4C-9 or 4C-10 for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance of defined in Section 1A.13.

Conclusion:

Warrant 9 is not met.

There is no Grade Crossing.

Study Conclusions:

The intersection of NM 599/ Via Veteranos/ NM 599 West Frontage Road Connector met zero warrants based on 2015 traffic counts. It is recommended that a traffic signal not be installed at this time for this intersection. See Appendix B for Signal Warrant Results.

Appendix A

Turning Movement

Traffic Counts

NM 599 & Via Veteranos & W. FR connector

Conducted by : Kyle B. Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015

Page No : 1

Groups Printed- Unshifted - Bank 1

			NM 59 om No					Veter	anos	iteu- Oi			NM 59					R con	nector		
Start Time	Right	Thru	Left		App Total	Right	Thru	Left		App Total	Right	Thru	Left		App Total	Right	Thru	Left		App Total	Int. Tota
03:00 PM	0	80	19	0	99	17	7	8	0	32	14	89	4	0	107	2	7	1	0	10	248
03:15 PM	4	112	17	ō	133	12	5	7	ō	24	8	72	4	ō	84	5	6	Ó	ő	11	252
03:30 PM	Ó	92	22	ō	114	19	8	9	ō	36	10	81	1	Ö	92	6	6	1	ő	13	255
03:45 PM	3	98	11	Ŏ	112	20	9	4	ō	33	2	106	3	ŏ	111	3	1	1	ő	5	261
Total	7	382	69	0	458	68	29	28	0	125	34	348	12	Ō	394	16	20	3	0	39	1016
04:00 PM	2	115	25	0	142	12	5	6	0	23	7	89	3	0	99	0	7	2	0	9	273
04:15 PM	4	125	20	0	149	21	4	4	0	29	6	93	5	0	104	6	3	3	0	12	294
04:30 PM	8	146	33	0	187	30	8	6	0	44	11	127	9	0	147	10	11	6	Ō	27	405
04:45 PM	9	143	30	0	182	28	5	9	0	42	13	131	8	0	152	9	13	7	Ō	29	405
Total	23	529	108	0	660	91	22	25	0	138	37	440	25	0	502	25	34	18	0	77	1377
05:00 PM	8	137	33	0	178	38	19	17	0	74	20	126	17	0	163	7	15	9	0	31	446
05:15 PM	4	116	17	0	137	18	6	6	0	30	5	93	3	0	101	4	4	3	0	11	279
05:30 PM	5	101	15	0	121	14	3	3	0	20	5	82	4	0	91	5	2	3	0	10	242
05:45 PM	3	68	9	0	80	8	2	4	0	14	4	61	7	0	72	3	3	- 5	0	11	177
Total	20	422	74	0	516	78	30	30	0	138	34	362	31	0	427	19	24	20	0	63	1144
Grand Total	50	1333	251	0	1634	237	81	83	0	401	105	1150	68	0	1323	60	78	41	0	179	3537
Apprch %	3.1	81.6	15.4	0		59.1	20.2	20.7	0		7.9	86.9	5.1	0		33.5	43.6	22.9	0		
Total %	1.4	37.7	7.1	0	46.2	6.7	2.3	2.3	0	11.3	3	32.5	1.9	0	37.4	1.7	2.2	1.2	0	5.1	
Unshifted	50	1333	251	0	1634	237	81	83	0	401	105	1150	68	0	1323	60	78	41	0	179	3537
% Unshifted	100	100	100	0	100	100	100	100	0	100	100	100	100	0	100	100	100	100	0	100	100
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(

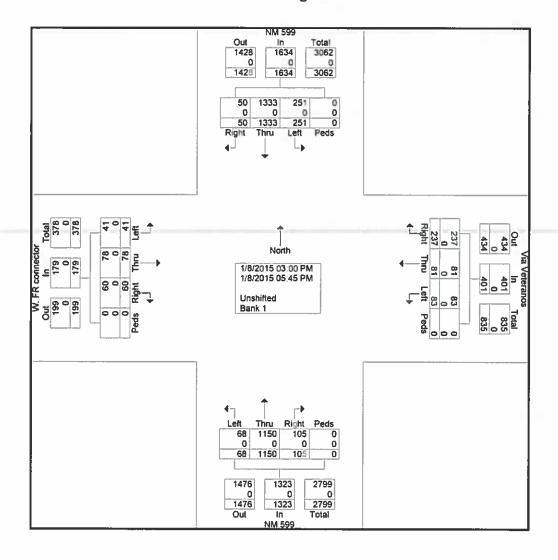
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015



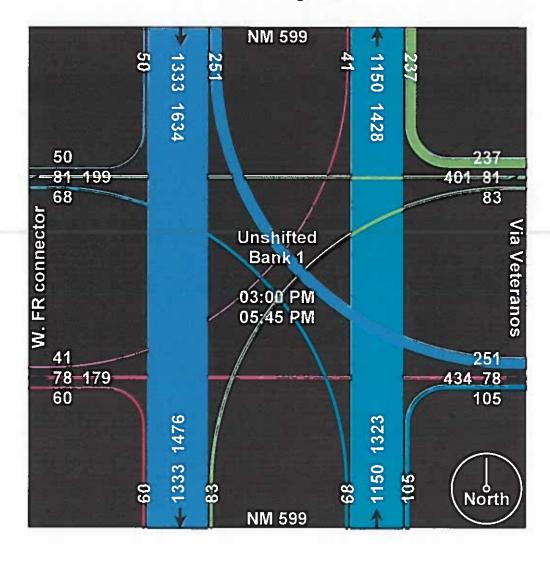
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name : nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015



NM 599 & Via Veteranos & W. FR connector

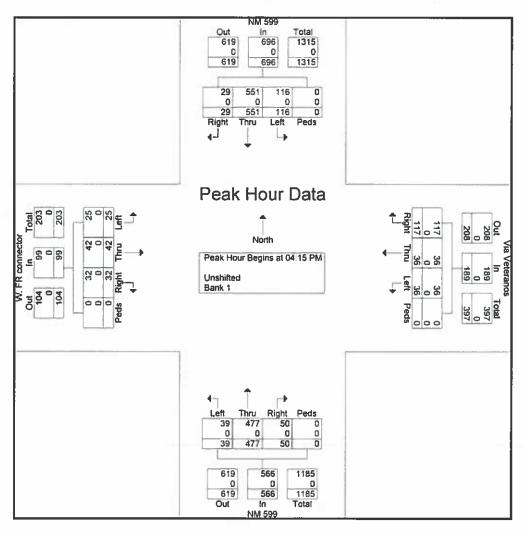
Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015

_			NM 59 om No	-				Veter rom E					NM 59 om Sc	_				R con rom W	nector 'est	•	
Start Time	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Int. Total
Peak Hour Ai							k 1 of	1										5.3516			
Peak Hour fo	r Entire	Inters	ection	Begins	at 04:1	5 PM															
04:15 PM	4	125	20	0	149	21	4	4	0	29	6	93	5	0	104	6	3	3	0	12	294
04:30 PM	8	146	33	0	187	30	8	6	0	44	11	127	9	0	147	10	11	6	0	27	405
04:45 PM	9	143	30	0	182	28	5	9	0	42	13	131	8	0	152	9	13	7	0	29	405
05:00 PM	8	137	33	0	178	38	19	17	0	74	20	126	17	0	163	7	15	9	0	31	446
Total Volume	29	551	116	0	696	117	36	36	0	189	50	477	39	0	566	32	42	25	0	99	1550
% App. Total	4.2	79.2	16.7	0		61.9	19	19	0		8.8	84.3	6.9	0		32.3	42.4	25.3	0		
PHF	.806	.943	.879	.000	.930	.770	.474	.529	.000	.639	.625	.910	574	.000	868	.800	.700	.694	.000	.798	.869
Unshifted % Unshifted	29	551	116	0	696	117	36	36	0	189	50	477	39	0	566	32	42	25	0	99	1550
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



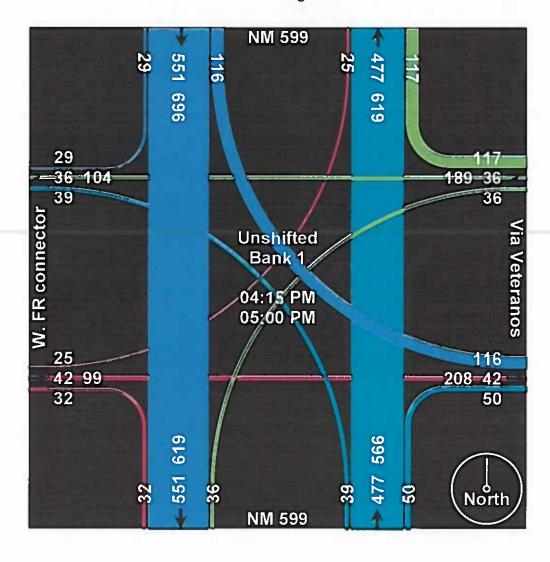
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015



NM 599 & Via Veteranos & W. FR connector

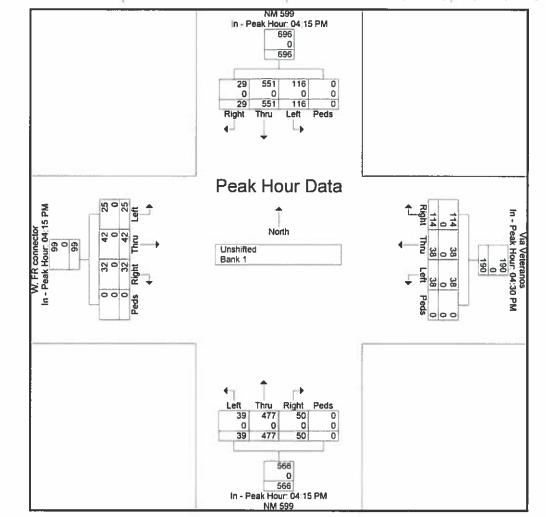
Conducted by : Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015

			NM 59 om No	-				Veter om E					NM 59 om So	_				R con	nector est		
Start Time	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Int. Total
Peak Hour A	nalysis	From (03:00 F	M to 0	5.45 PN	/I - Peal	k 1 of 1	-					17	No.							artice products a
Peak Hour fo	r Each	Approa	ach Be	gins at:																	
	04:15 PM			2007		04:30 PM					04:15 PN	1				04:15 PN	t.				
+0 mins.	4	125	20	0	149	30	8	6	0	44	6	93	5	0	104	6	3	3	0	12	
+15 mins.	8	146	33	0	187	28	5	9	0	42	11	127	9	0	147	10	11	6	0	27	
+30 mins.	9	143	30	0	182	38	19	17	0	74	13	131	8	0	152	9	13	7	0	29	
+45 mins.	8	137	33	0	178	18	6	6	0	30	20	126	17	0	163	7	15	9	0	31	
Total Volume	29	551	116	0	696	114	38	38	0	190	50	477	39	0	566	32	42	25	0	99	
% App. Total	4.2	79.2	16.7	0		60	20	20	0		8.8	84.3	6.9	0		32.3	42.4	25.3	0		
PHF	.806	943	.879	.000	.930	.750	.500	.559	.000	.642	.625	.910	.574	.000	.868	800	.700	.694	.000	.798	
Unshifted % Unshifted	29	551	116	0	696	114	38	38	0	190	50	477	39	0	566	32	42	25	0	99	
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	



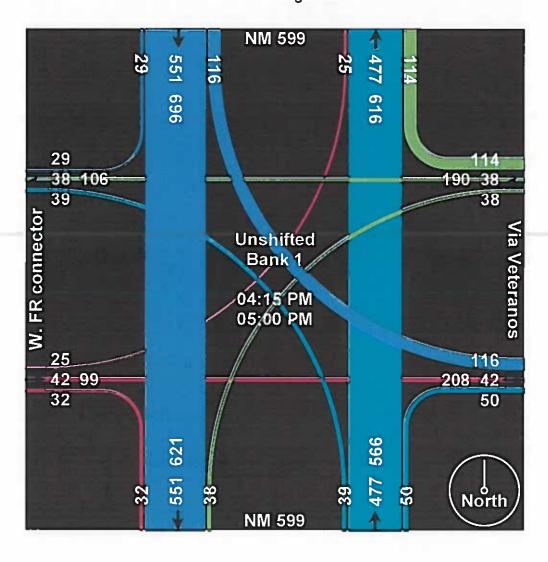
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015



NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector

Site Code : 00010815 Start Date : 1/8/2015



NM 599 & Via Veteranos & W. FR connector

Conducted by : Kyle B. Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015

Groups Printed- Unshifted - Bank	: 1.	

			NM 59 om Ne	orth		Comment.		Veter	anos ast	inted* Of		Fı	NM 59 om Sc	outh				rom V			
Start Time	Right	Thru	Left		App Total	Right	Thru	Left	1	App Total	Right		Left		App Total	Right	Thru	Left	Peds	App Total	Int. Total
07:00 AM	0	75	16	0	91	7	2	4	0	13	7	126	0	0	133	2	2	2	0	6	243
07:15 AM	0	80	14	Ð	94	12	0	7	0	19	12	162	2	0	176	0	6	2	0	8	297
07:30 AM	0	106	19	0	125	18	1	5	0	24	26	247	1	0	274	3	5	6	0	14	437
07:45 AM	1	140	36	0	177	12	4	14	0	30	51	271	10	0	332	7	6	0	0	13	552
Total	1	401	85	0	487	49	7	30	0	86	96	806	13	0	915	12	19	10	0	41	1529
08:00 AM	2	108	31	0	141	25	4	8	0	37	23	212	4	0	239	2	5	2	0	9	426
08:15 AM	1	90	21	0	112	25	4	8	0	37	15	178	1	0	194	1	11	1	0	13	356
08:30 AM	1	84	12	0	97	12	6	4	0	22	19	110	5	0	134	4	7	0	0	11	264
08:45 AM	0	77	21	0	98	14	10	2	0	26	9	128	10	0	147	1	4	2	0	7	278
Total	4	359	85	0	448	76	24	22	0	122	66	628	20	0	714	8	27	5	0	40	1324
09:00 AM	2	54	12	0	68	17	2	9	0	28	9	83	2	0	94	2	9	1	0	12	202
09:15 AM	- 1	58	14	0	73	17	5	6	0	28	5	78	9	0	92	2	8	1	0	11	204
09:30 AM	0	63	15	0	78	6	5	1	0	12	3	89	2	0	94	1	6	2	0	9	193
09:45 AM	1	61	10	0	72	10	- 6	3	0	19	4	73	2	0	79	5	8	1	0	14	184
Total	4	236	51	0	291	50	18	19	0	87	21	323	15	0	359	10	31	5	0	46	783
** BREAK **	*																				
11:00 AM	1	100	18	0	119	20	2	5	0	27	5	106	3	0	114	1	7	2	0	10	270
11:15 AM	i	84	14	ō	99	7	6	7	ō	20	7	84	2	ō	93	3	9	1	ō	13	225
11:30 AM	1	1	20	Ö	22	14	5	5	ō	24	Ö	0	2	õ	2	5	4	Ö	ō	9	57
11:45 AM	3	75	17	ō	95	13	2	5	ō	20	5	67		ō	73	5	5	2	ŏ	12	200
Total	6	260	69	0	335	54	15	22	0	91	17	257	8	0	282	14	25	5	Ō	44	752
12:00 PM	1	86	14	0	101	15	8	9	٥	32	4	76	2	0	82	3	9	2	0	14	229
12:15 PM	1	97	14	ō	112	9	5	2	ō	16	5	88	3	ō	96	3	4	1	Ď	8	232
12:30 PM	1	77	22	ō	100	13	6	3	ō	22	6	82	6	Ö	94	2	10	i	ō	13	229
12:45 PM	3	85	15	ō	103	21	12	4	ō	37	7	95	Ť	ō	103	2	11	2	ō	15	258
Total	6	345	65	0	416	58	31	18	Ö	107	22	341	12	0	375	10	34	6	ő	50	948
01:00 PM	1	69	15	0	85	20	6	2	0	28	8	51	3	0	62	3	6	1	0	10	185
01:15 PM	1	69	16	ō	86	9	4	4	ō	17	4	58	3	ŏ	65	ō	8	2	Ö	10	178
01:30 PM	1	68	20	ŏ	89	14	6	9	Ö	29	8	52	3	ő	63	2	2	1	Ö	5	186
01:45 PM	2	67	15	ŏ	84	20	6	3	0	29	8	45	3	ŏ	56	1	5	2	ő	8	177
Total	5	273	66	0	344	63	22	18	0	103	28	206	12	0	246	6	21	6	0	33	726
Grand Total	26	1874	421	0	2321	350	117	129	0	596	250	2561	80	0	2891	60	157	37	0	254	6062
Apprch %	1.1	80.7	18.1	ő		58.7	19.6	21.6	Ö	204	8.6	88.6	2.8	ő		23.6	61.8	14.6	ő		2002
Total %	0.4	30.9	6.9	ŏ	38.3	5.8	1.9	2.1	o o	9.8	4.1	42.2	1.3	ŏ	47.7	1	2.6	0.6	0	4.2	
Unshifted	26	1874	421	ō	2321	350	117	129	ő	596	250	2561	80	0	2891	60	157	37	0	254	6062
% Unshifted	100	100	100	Ö	100	100	100	100	ŏ	100	100	100	100	0	100	100	100	100	0	100	100
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	ŏ	0	0	0	_	_	0	0	0	_	0
% bank 1	U	U	U	U	U	U	U	U	U	U	U	U	U	U	0	0	U	0	O	0	

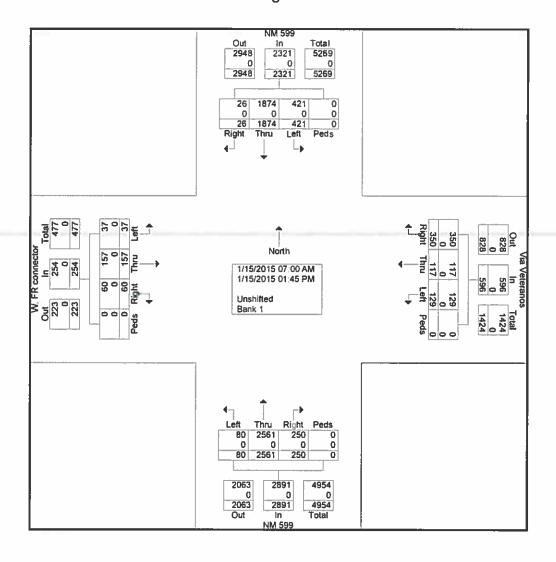
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015



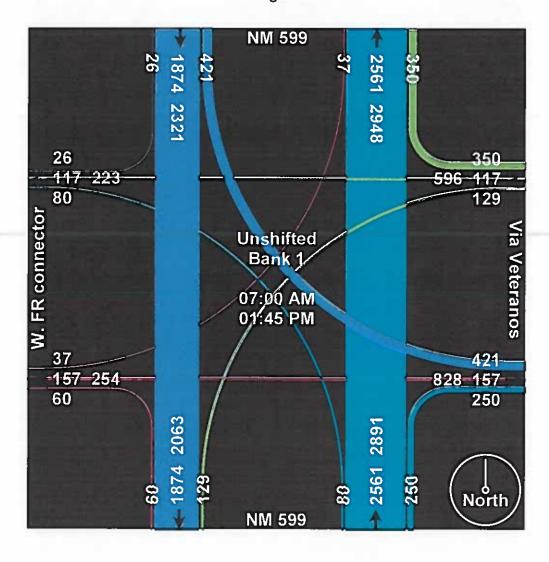
NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015



NM 599 & Via Veteranos & W. FR connector

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Conducted by: Kyle B.

Weather: Clear

% Unshifted

Bank 1

% Bank 1

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015

Page No : 4

			NM 59 om No					Veter					NM 59 om Sc	_				R con	nector est	•	
Start Time	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App. Total	Int. Tota
Peak Hour A	nalysis	From	07:00 /	AM to (9:45 A	1 - Pea	k 1 of	1													
Peak Hour fo	r Entire	Inters	ection	Begins	at 07:3	MA 0															
07:30 AM	0	106	19	0	125	18	1	5	0	24	26	247	1	0	274	3	5	6	0	14	43
07:45 AM	1	140	36	0	177	12	4	14	0	30	51	271	10	0	332	7	6	ō	ō	13	55
08:00 AM	2	108	31	0	141	25	4	8	0	37	23	212	4	ō	239	2	5	2	ō	9	42
08:15 AM	1	90	21	0	112	25	4	8	Ō	37	15	178	1	Ö	194	1	11	$-\tilde{1}$	ŏ	13	35
Total Volume	4	444	107	0	555	80	13	35	0	128	115	908	16	0	1039	13	27	9	n	49	177
% App. Total	0.7	80	19.3	0		62.5	10.2	27.3	ō		11.1	B7.4	1.5	0		26.5	55.1	18.4	0		
PHF	500	793	743	.000	.784	800	.813	625	.000	865	.564	.838	400	.000	.782	464	.614	.375	.000	875	.80
Unshifted % Unshifted	4	444	107	0	555	80	13	35	0	128	115	908	16	0	1039	13	27	9	0	49	177
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
eak Hour A eak Hour fo		Appro				/I - Pea		1			07 30 AM					07.30 AM					
+0 mins	0.3074	106	19	0	125	18	1	5	0	24	26	247	1	0	274	3	5	6	0	14	
+15 mins.	1	140	36	ő	177	12	À	14	ő	30	51	271	10	ő	332	7	6	Ö	Ŏ	13	
+30 mins.	2	108	31	Õ	141	25	4	8	Õ	37	23	212	4	Ď	239	2	5	2	0	9	
+45 mins.	1	90	21	0	112	25	<i>A</i>	8	ŏ	37	15	178	1	0	194	1	11	4	0	13	
Total Volume	4	444	107	0	555	80	13	35	0	128	115	908	16	0	1039	13	27	9	0	49	
% App. Total	0.7	80	19.3	0	333	62.5	10.2	27.3	0	120	11.1	87.4	1.5	0	1009	26.5	55.1	18.4	0	45	
PHF	.500	.793	.743	.000	784	.800	.813	.625	.000	.865	.564	.838	.400	.000	.782	.464	.614	.375	.000	.875	
Unshifted	4	444	107	0	555	80	13	35	0	128	115	908	16	.000	1039	13	27	.3/3	0	49	

NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015

Page No : 5

			NM 59 om No	_				Veter					NM 59 om So	-				R cons	nector est		
Start Time	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Right	Thru	Left	Peds	App Total	Int. Total
Peak Hour Ar	nalysis	From 1	10:00 A	M to 0	1:45 PN	1 - Pea	k 1 of 1	1		10892100		-29119 3	VICUITY OF								
Peak Hour for	r Entire	Inters	ection	Begins	at 12:0	0 PM															
12:00 PM	1	86	14	0	101	15	8	9	0	32	4	76	2	0	82	3	9	2	0	14	229
12:15 PM	1	97	14	0	112	9	5	2	0	16	5	88	3	- 0	96	- 3	4	1	0	8	232
12:30 PM	1	77	22	0	100	13	6	3	0	22	6	82	6	0	94	2	10	1	0	13	229
12:45 PM	3	85	15	0	103	21	12	4	0	37	_ 7	95	1	0	103	2	11	2	0	15	258
Total Volume	6	345	65	0	416	58	31	18	0	107	22	341	12	0	375	10	34	6	0	50	948
% App. Total	1.4	82.9	15.6	0		54.2	29	16.8	0		5.9	90.9	3.2	0		20	68	12	0		
PHF	.500	.889	.739	.000	929	690	.646	.500	.000	.723	.786	897	.500	.000	.910	.833	.773	.750	.000	.833	.919
Unshifted % Unshifted	6	345	65	0	416	58	31	18	0	107	22	341	12	0	375	10	34	6	0	50	948
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	٥	0	0	0	0	0	0	0	0	0	0

Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1

	12:00 PM					12:45 PM	1				12:00 PM					12:00 PM				
+0 mins.	1	86	14	0	101	21	12	4	0	37	4	76	2	0	82	3	9	2	0	14
+15 mins.	1	97	14	0	112	20	6	2	0	28	5	88	3	٥	96	3	4	1	٥	8
+30 mins.	1	77	22	0	100	9	4	4	0	17	6	82	6	0	94	2	10	1	0	13
+45 mins.	3	85	15	0	103	14	6	9	0	29	7	95	1	0	103	2	11	2	0	15
Total Volume	6	345	65	0	416	64	28	19	0	111	22	341	12	0	375	10	34	6	0	50
% App. Total	1.4	82.9	15.6	0		57.7	25.2	17.1	0		5.9	90.9	3.2	0	5.47.60	20	68	12	0	
PHF	.500	889	.739	.000	929	.762	.583	.528	.000	.750	.786	.897	500	.000	910	833	.773	750	000	.833
Unshifted % Unshifted	6	345	65	0	416	64	28	19	0	111	22	341	12	0	375	10	34	6	0	50
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	Ω	0	D	Ô	0

NM 599 & Via Veteranos & W. FR connector

Conducted by: Kyle B.

Weather: Clear

File Name: nm 599 & via veteranos & w. fr connector # 2

Site Code : 00011515 Start Date : 1/15/2015



Appendix B

Highway Capacity Software

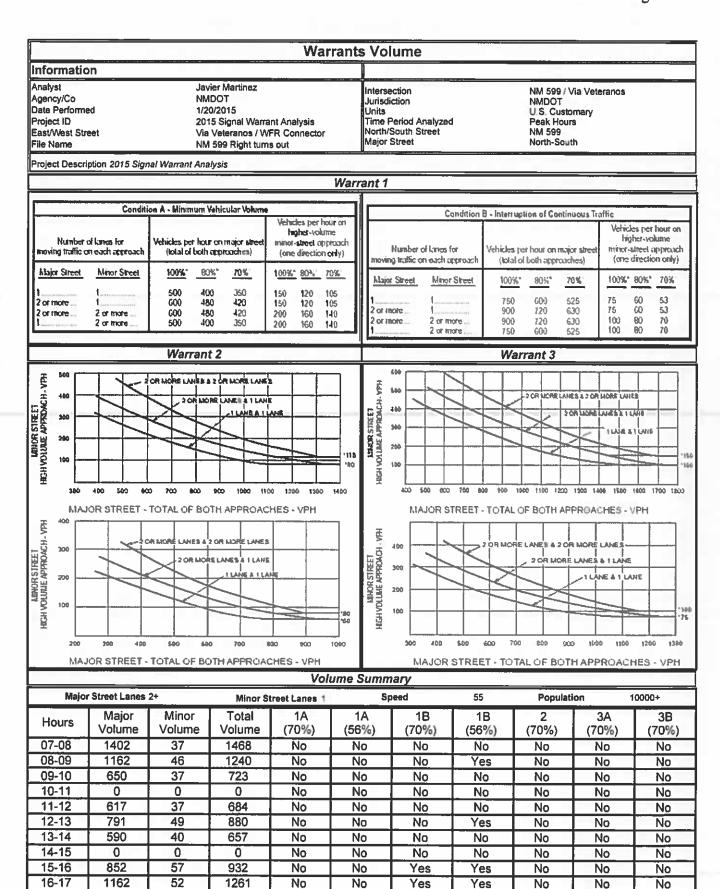
Warrant Results

Warrants Summary Page 1 of 2

				Warra	ants :	Summ	ary					T	
Information													
Analyst Agency/Co Date Performed Project ID East/West Street File Name	NI 1/ 20 Ar Vi Co	MDOT Intersection NM 599 / Via Vete 20/2015 Jurisdiction NMDOT Units U.S. Customary halysis Time Period Analyzed Peak Hours a Veteranos / WFR North/South Street North-South M 599 Right turns out								nos			
Project Description 2015	Sign	al Wai	rant A	nalysis									
General			Roadway Network										
Major Street Speed (mph)	55			pulation				1-	o Major		s		
Nearest Signal (ft)	9999	2	Coordinated Signal System Weekend Count							_			
Crashes (per year)	1	Y		equate T	rials o	f Altern	atives	5-у	r Growt	h Facto	or		0
Geometry and Traffic			EB	T 57		WB	DT		NB			SB	L 5.7
Number of James Al		LT	TH	RT	LT	TH	RT	LT	TH	RT 1	LT 1	TH 2	RT
Number of lanes, N Lane usage		0	1 LT	10	0	1 LT	0	1 L	2 T	R	L	T	1 R
Vehicle Volume Average (vph)	es	6	19	0	17	16	0	12	309	29	56	267	6
Peds (ped/h) / Gaps (gaps/h)			0/0	_		0/0			0/0	_		0/0	
Delay (s/veh) / (veh-hr)		-	0/0	II _		0/0			0/0	_		0/0	
Warrant 1: Eight-Hour	ight-Hour Vehicular Volume												
1 A. Minimum Vehicular	Volu	nes (E	oth m	ajor appi	roache	esand	highe	r mino	· · · · · · · · · · · · · · · · · · ·				
					r approachesand higher minor approach)or								
					h majo	г аррго	aches -	-and	higher minor approach)				
Warrant 2: Four-Hour													
	Vehicular Volumes (Both major approachesand higher minor approach)												
Warrant 3: Peak Hour								_					
3 A. Peak-Hour Condition													
3 B. Peak- Hour Vehicul			(Both	major ap	ргоас	hesa	nd hig	her m	nor app	oroach)			
Warrant 4: Pedestrian													
4 A. Four Hour Volumes		-											
4 B. One-Hour Volumes													
Warrant 5: School Cro													
5. Student Volumesar	1d		_							_			
5. Gaps Same Period					***								
	Varrant 6: Coordinated Signal System . Degree of Platooning (Predominant direction or both directions)												
	Varrant 7: Crash Experience												
	A. Adequate trials of alternatives, observance and enforcement failed —and—							√					
·	eported crashes susceptible to correction by signal (12-month period)and												

7 C. 80% Volumes for Warrants 1A, 1Bor 4 are satisfied	
Warrant 8: Roadway Network	
8 A. Weekday Volume (Peak hour totaland projected warrants 1, 2 or 3)or	
8 B. Weekend Volume (Five hours total)	
Warrant 9: Grade Crossing	
9 A. Grade Crossing within 140 ftand	
9 B. Peak-Hour Vehicular Volumes	
Convicted # 2010 Heliconity of Clarks All Digits Personal	410010045 7.40 44

Warrants Volume



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60

0

415

943

0

8169

17-18

18-19

Totals

HCS+TM Version 5.5

Yes

No

5

Yes

No

1

Yes

Νo

3

Generated: 1/23/2015 7:49 AM

No

No

0

1047

0

8892

No

No

0

No

No

0

No

No

Appendix C

Crash History



3 Year Crash Data

NM 599 & Via Veteranos

YEAR	
2011	3
2012	2
2013	2
Total	7

Notes:

Source: 2002-2012 NMDOT Crash File, as of 07/17/2014. 2013 Crash File, as of

11/12/14 is Preliminary.

NMDOT/ TRU

CRASH REPORT NUMBER	CRASH DATE	TIME OF CRASH	COUNTY	CRASH LOCATION (CITY OR RURAL)
23271516	6/5/2011	1625	Santa Fe	Rural (Non-Urban)
23158743	6/17/2011	1130	Santa Fe	Rural (Non-Urban)
23158460	7/26/2011	658	Santa Fe	Rural (Non-Urban)

PRIMARY STREET	SECONDARY STREET	ROUTE NAME	MILE POST
NM 599	CR 70 CONNECTOR		
NM 599	CAMINO LA TIERRA	NM 599	9.799995422
NM 599		NM 599	9.099998474

REPORTING	NUMBER OF	NUMBER OF
AGENCY	PEOPLE IN CRASH	PEOPLE KILLED
County Sheriff	7	0
County Sheriff	2	0
County Sheriff	2	0

NUMBER OF PEOPLE WITH INCAPACITATING INJURIES	NUMBER OF PEOPLE WITH VISIBLE INJURIES
	2
()
(0

NUMBER OF PEOPLE WITH POSSIBLE INJURIES	NUMBER OF PEOPLE NOT INJURED
0	5
0	1
2	0

NUMBER OF VEHICLES, BICYCLISTS, ETC. INVOLVED		WEATHER	LIGHTING	CRASH SEVERITY
	2 (Clear	Daylight	Injury Crash
	2 (Clear	Daylight	Injury Crash
	1 (Clear	Daylight	Injury Crash

CRASH CLASSIFICATION Other Vehicle

HIGHEST CONTRIBUTING FACTOR TO CRASH Improper Turn

CRASH ANALYSIS

Other Vehicle Other Vehicle Overturn

Improper Turn
None
Excessive Speed

Sd-Rear End Sd-Sideswipe Overturn-Right

Crash Report Number	Crash date	Time of Crash	County	Urban/Rural
23350543	3/28/2012	15:50	SANTA FE	U
30059861	12/5/2012	15:02	SANTA FE	U

City	Primary Street	Second Street	Location/Landmark
	NM 599	MILE POST 9	
SANTA FE	NM 599	NONE	

Route	Milepost	Reporting Agency	Fatal/Injury
NM 599	9	SANTA FE COUNTY SHERIFFS OFFICE	-
NM 599S	9	NEW MEXICO STATE POLICE (NMSP)	INJURY

Numbe Peop Kille	27.4	ber of ople ured	NUMBER OF PEOPLE WITH INCAPACITATING INJURIES
0	0	0	
0	4	0	

NUMBER OF PEOPLE WITH VISIBLE INJURIES	NUMBER OF PEOPLE WITH POSSIBLE INJURIES	NUMBER OF PEOPLE NOT INJURED
0	0	1
0	4	0

Total Number of People in Crash	Analysis Code	Analysis	Crash Classification
1	16	1016	FIXED OBJECT
4	8	408	OTHER VEHICLE

Hit Run	Lighting	Number of Vehicles	Road Character	Road Grade	Weather	рТуре
N	DAYLIGHT	1	STRAIGHT	LEVEL	CLEAR	GUARDRAIL
N	DAYLIGHT	2	STRAIGHT	LEVEL	CLEAR	

Crash Report Number	Crash Date	Time of Crash	County	Urban/ Rurl	City
233719411	12/3/2013	11:37	SANTA FE	R	Left Bl

Primary Street	Secondary Street	Location/Landmark	Route	Mile Post
NM 599 NORTH BOUND		NM 599 @ MP 8	NM 599	8

Reporting Agency	Fatal/Injury	Number Of People Killed
SANTA FE COUNTY SHERIFFS OFFICE	INJURY	0

Number of People Injured

1

NUMBER OF PEOPLE WITH INCAPACITATING INJURIES

n

NUMBER OF PEOPLE WITH VISIBLE INJURIES

NUMBER OF PEOPLE WITH POSSIBLE INJURIES

0

1

NUMBER OF PEOPLE NOT INJURED	Total Number people in crash	Analysis
_		

2

3 SD-BOTH STRAIGHT

Analysis Name	CrashClassification	Hit and Run
08 BOTH GOING STRAIGHT/FROM SAME DIR	OTHER VEHICLE	V